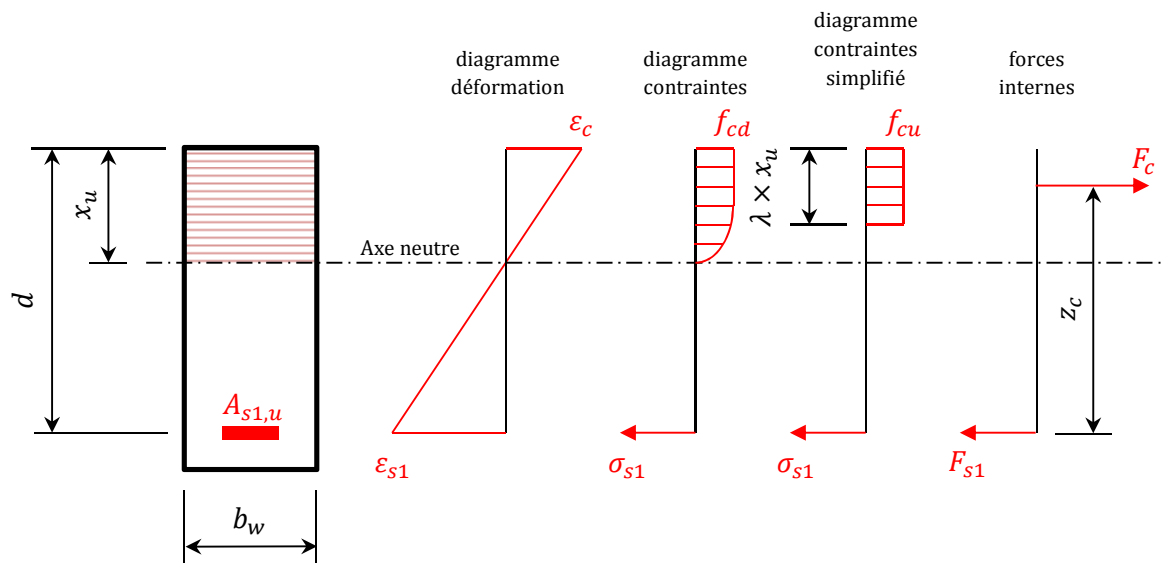


4.1 - FLEXION SIMPLE - ELU



$$f_{cd} = \alpha_{cc} \times \frac{f_{ck}}{\gamma_c} \quad \text{avec} \quad \alpha_{cc} = 1 \text{ (valeur annexe nationale)}$$

$$f_{cu} = \eta \times f_{cd} \quad \text{avec} \quad \begin{array}{ll} \eta = 1 & \text{si } f_{ck} \leq 50 \text{ MPa} \\ \eta = 1 - \frac{f_{ck}-50}{200} & \text{si } 50 \text{ MPa} < f_{ck} \leq 90 \text{ MPa} \end{array}$$

$$\begin{array}{ll} \lambda = 0.8 & \text{si } f_{ck} \leq 50 \text{ MPa} \\ \lambda = 0.8 - \frac{f_{ck}-50}{400} & \text{si } 50 \text{ MPa} < f_{ck} \leq 90 \text{ MPa} \end{array}$$

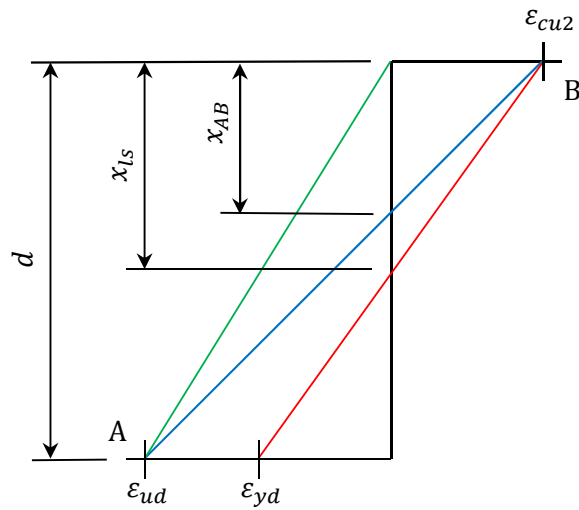
$$\begin{array}{l} x_u = \alpha_u \times d \\ z_c = d - \frac{\lambda \times x_u}{2} = d \times \left(1 - \frac{\lambda \times \alpha_u}{2}\right) \end{array}$$

$$\begin{array}{l} F_c = b_w \times \lambda \times x_u \times f_{cu} = b_w \times \lambda \times \alpha_u \times d \times f_{cu} \\ F_{s1} = A_{s1,u} \times \sigma_{s1} \end{array}$$

Equations d'équilibre :

$$\begin{array}{l} M_{Ed} = F_c \times z_c = b_w \times \lambda \times \alpha_u \times d \times f_{cu} \times d \times \left(1 - \frac{\lambda \times \alpha_u}{2}\right) = b_w \times d^2 \times f_{cu} \times \lambda \times \alpha_u \times \left(1 - \frac{\lambda \times \alpha_u}{2}\right) \\ \text{on pose :} \quad \mu_{cu} = \frac{M_{Ed}}{b_w \times d^2 \times f_{cu}} = \lambda \times \alpha_u \times \left(1 - \frac{\lambda \times \alpha_u}{2}\right) \text{ (moment réduit)} \end{array}$$

$$\begin{array}{l} M_{Ed} = F_{s1} \times z_c = A_{s1,u} \times \sigma_{s1} \times z_c \\ \rightarrow A_{s1,u} = \frac{M_{Ed}}{\sigma_{s1} \times z_c} \end{array}$$



Calcul de μ_{AB} :

$$\epsilon_c = \epsilon_{cu2}$$

$$\epsilon_{s1} = \epsilon_{ud}$$

$$\alpha_{AB} = \frac{x_{AB}}{d} = \frac{\epsilon_{cu2}}{\epsilon_{ud} + \epsilon_{cu2}}$$

$$\mu_{AB} = \lambda \times \alpha_{AB} \times \left(1 - \frac{\lambda \times \alpha_{AB}}{2}\right)$$

Calcul de μ_{ls} :

$$\epsilon_c = \epsilon_{cu2}$$

$$\epsilon_{s1} = \epsilon_{yd}$$

$$\alpha_{ls} = \frac{x_{ls}}{d} = \frac{\epsilon_{cu2}}{\epsilon_{yd} + \epsilon_{cu2}}$$

$$\mu_{ls} = \lambda \times \alpha_{ls} \times \left(1 - \frac{\lambda \times \alpha_{ls}}{2}\right)$$

si $\mu_{cu} > \mu_{ls}$ → Aciers comprimés

si $\mu_{cu} \leq \mu_{ls}$ → Pas d'aciers comprimés
 si $\mu_{cu} \leq \mu_{AB}$ → Pivot A
 si $\mu_{cu} > \mu_{AB}$ → Pivot B

Cas sans aciers comprimés $\mu_{cu} \leq \mu_{ls}$:

$$\mu_{cu} = \lambda \times \alpha_u \times \left(1 - \frac{\lambda \times \alpha_u}{2}\right)$$

→ Equation du 2nd degré en α_u

$$\rightarrow \text{Solution : } \alpha_u = \frac{1}{\lambda} \left(1 - \sqrt{1 - 2 \times \mu_{cu}}\right)$$

si $\mu_{cu} \leq \mu_{AB}$ → Pivot A :

$$\varepsilon_c < \varepsilon_{cu2}$$

$$\varepsilon_{s1} = \varepsilon_{ud}$$

si $\mu_{cu} > \mu_{AB}$ → Pivot B :

$$\varepsilon_c = \varepsilon_{cu2}$$

$\varepsilon_{yd} < \varepsilon_{s1} < \varepsilon_{ud}$ (ε_{s1} est sur le palier du diagramme contraintes/déformations)

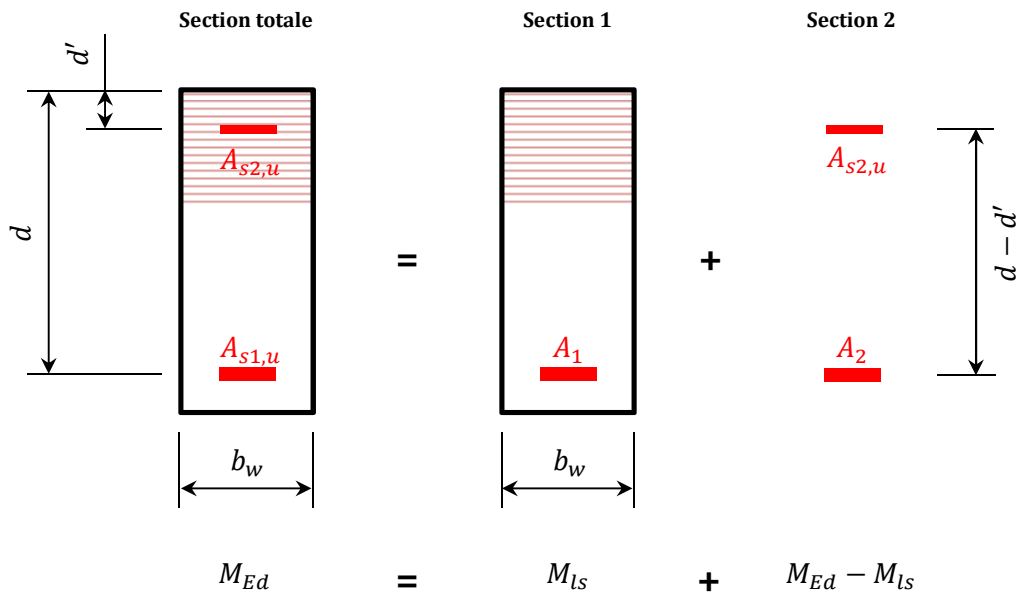
$$\varepsilon_{s1} = \varepsilon_{cu2} \times \left(\frac{1 - \alpha_u}{\alpha_u}\right)$$

$$\sigma_{s1} = A + B \times \varepsilon_{s1} \text{ (Equation de la droite du palier incliné)}$$

$$z_c = d \times \left(1 - \frac{\lambda \times \alpha_u}{2}\right)$$

$$A_{s1,u} = \frac{M_{Ed}}{\sigma_{s1} \times z_c}$$

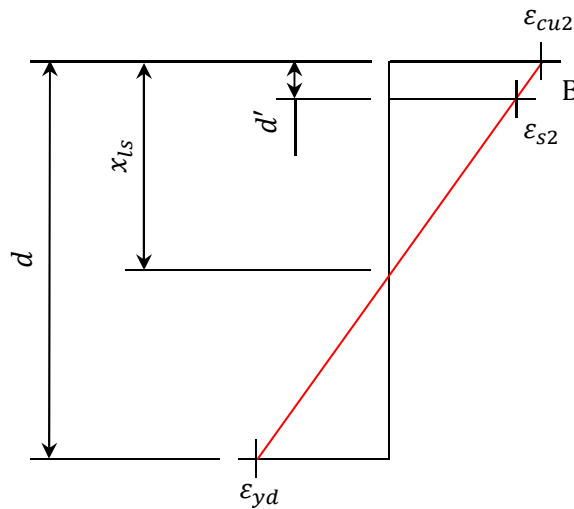
Cas avec aciers comprimés $\mu_{cu} > \mu_{ls}$:



$$M_{ls} = \mu_{ls} \times b_w \times d^2 \times f_{cu}$$

Calcul de $A_{s2,u}$:

$$M_{Ed} - M_{ls} = A_{s2,u} \times \sigma_{s2} \times (d - d')$$



$$\epsilon_{s2} = \epsilon_{cu2} \times \frac{\alpha_{ls} \frac{d'}{d}}{\alpha_{ls}}$$

si $\epsilon_{s2} \leq \epsilon_{yd}$: $\sigma_{s2} = E_s \times \epsilon_{s2}$ (droite de Hooke)

si $\epsilon_{s2} > \epsilon_{yd}$: $\sigma_{s2} = A + B \times \epsilon_{s2}$ (palier incliné)

$$A_{s2,u} = \frac{M_{Ed} - M_{ls}}{(d - d') \times \sigma_{s2}}$$

Calcul de $A_{s1,u}$:

$$A_{s1,u} = A_1 + A_2$$

$$A_1 = \frac{M_{ls}}{\sigma_{s1} \times z_c} \quad \text{avec} \quad \sigma_{s1} = f_{yd}$$

$$z_c = d \times \left(1 - \frac{\lambda \times \alpha_{ls}}{2}\right)$$

$$M_{Ed} - M_{ls} = A_{s2,u} \times \sigma_{s2} \times (d - d') = A_2 \times \sigma_{s1} \times (d - d')$$

$$A_{s2,u} \times \sigma_{s2} = A_2 \times \sigma_{s1}$$

$$A_2 = A_{s2,u} \times \frac{\sigma_{s2}}{\sigma_{s1}}$$

$$A_{s1,u} = \frac{M_{ls}}{\sigma_{s1} \times z_c} + A_{s2,u} \times \frac{\sigma_{s2}}{\sigma_{s1}}$$

Flexion simple - ELU

Section rectangulaire

(diagramme contraintes-déformations des aciers avec palier incliné)

Béton	Acier	Dimensions	Sollicitations
f_{ck}	f_{yk}	b_w	M_{Ed}
γ_c	γ_s	d	
α_{cc}	E_s	d'	
	Classe de ductilité		

